

1a) a) To design for safety

b) The design must be economical

c) The deformation of the structure must not impair with the integrity of the structure

1b) Limit state design considers the disadvantages of load factor design and any other failure that can cause the structure to be structurally unfit while Elastic design is a method of analysis which the design of a structural member is based on a linear stress-strain relationship, assuming that the working stress are only a fraction of the elastic limit of the material

STAIR CASE Design

$$\text{slope Factor} = \frac{\sqrt{R^2 + T^2}}{T} = \frac{\sqrt{150^2 + 275^2}}{275} = 1.14$$

+ Load Analysis

$$\begin{aligned} \text{A. WAIST} &= R \times 24 \text{ kN/m}^2 \\ &= 0.15 \times 24 = 3.6 \text{ kN/m}^2 \end{aligned}$$

$$\text{B. FINISHES} = 1.2 \text{ kN/m}^2$$

$$\begin{aligned} \text{C. STEPS} &= T \times \frac{1}{2} \times 24 \text{ kN/m}^2 \\ &= 0.275 \times 0.5 \times 24 = 3.3 \text{ kN/m}^2 \end{aligned}$$

$$\begin{aligned} \text{D. G.K} &= (A+B) \times 3 + C \\ &= (4.8 \times 1.14) + 3.3 \\ &= 8.77 \text{ kN/m} \end{aligned}$$

$$\begin{aligned} \text{D.L, F} &= 1.4 \text{ G.K} + 1.6 \text{ Q.K} \\ &= 1.4 (8.77) + 1.6 (1.5) \\ &= 14.68 \text{ kN/m}^2 \end{aligned}$$

$$\text{Span} = T_{\text{total}} + 0.5(l_a + l_b) = (275 \times 12) + 0.5(225 + 225) = 3.525 \text{ m}$$

$$\begin{aligned} d &= h - \text{cover} - \frac{1}{2}\phi \\ &= 150 - 25 - 6 = 119 \text{ mm} \end{aligned}$$

$$M = \frac{F L^2}{10} = \frac{14.68 \times 3.525^2}{10} = 18.24 \text{ kNm}$$

$$K = \frac{M}{bd^2 f_{ck}} = \frac{18.24 \times 10^6}{1000 \times 119^2 \times 25} = 0.052$$

$$I_a = 0.5 + \sqrt{0.25 - \frac{K}{0.9}} = 0.5 + \sqrt{0.25 - \frac{0.052}{0.9}} = 0.938$$

$$z = I_a d = 0.938 \times 119 = 111.622 \text{ mm}$$

$$A_s = \frac{M}{0.95 f_y z} = \frac{18.24 \times 10^6}{0.95 \times 410 \times 111.622} = 419.53$$

$$A_{s, \text{prov}} = 452 \text{ mm}^2$$

Provide Y12 @ 259 c/c ($A_{s, \text{prov}} = 452 \text{ mm}^2$)

Deflection Check

$$f_s = \frac{2}{3} \times \frac{1}{\beta} \times \frac{A_{req}}{A_{prov}} \times f_y V$$

$$f_s = \frac{2}{3} \times 1 \times \frac{419.53}{452} \times 250$$
$$= 154.69 \text{ N/mm}^2$$

$$M.F = 0.56 + \frac{477 - 154.69}{120 \left(0.4 + \frac{18.24 \times 10^6}{1000 \times 119^2} \right)} = 1.78$$

$$d_{req} = \frac{\text{span}}{mf \times edr} = \frac{3525}{1.78 \times 26} = 76.17 \text{ mm}$$

since $d_{req} < d$, Deflection is OK

for 1st

$$2a) P_1 = P_2 = P_3 = \frac{4300}{4000} = 1.075 < 2 = \text{2 way slab}$$

$$P_7 = P_8 = P_9 = \frac{4500}{4000} = 1.125 < 2 = \text{2 way slab}$$

$$P_4 = P_5 = P_6 = \frac{4300}{4000} = 1.075 < 2 = \text{2 way slab}$$

$$P_{10} = P_{11} = P_{12} = \frac{4000}{1500} = 2.66 > 2 = \text{1 way slab}$$

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Designs for P2

$$\frac{L_y}{L_x} = \frac{4300}{4000} = 1.075 \approx 1.1$$

shortspan coefficient = $\frac{1}{0.054}$

longspan coefficient = $\frac{0.058}{0.044}$

Assuming specifications of slab thickness = 175mm
 $f_{cu} = 25 \text{ N/mm}^2$
 $f_y = 460 \text{ N/mm}^2$

$$DL = 1.4 G_k + 1.6 Q_k$$

$G_k =$ weight of slab = 0.175×24
Partition = 1.0
finishes = 1.2

 6.4 kN/m^2

Assuming for residential factory

$$DL = (1.4 \times 6.4) + (1.6 \times 5) = 16.96 \approx 17 \text{ kN/m}^2$$

$$\text{Short span coefficient} = \frac{0.044}{0.033}$$

$$\text{Long span coefficient} = \frac{0.037}{0.028}$$

Short span mid = P

$$M = \alpha_w l^2 w = 0.044 \times 17 \times 4^2 = 11.968$$

$$d = h - \text{cover} - \frac{1}{2} \phi = 144$$

$$k_s = \frac{M}{bd^2 k_{cs}} = \frac{11.968 \times 10^6}{1000 \times 144^2 \times 25} = 0.023$$

$$k = \beta_a = 0.5 + \sqrt{0.25 - \frac{k_s}{0.9}} = 0.97 > 0.95 = 0.95$$

$$z = \beta_a \cdot d = 0.95 \times 144 = 136.8$$

$$A_s = \frac{M}{0.95 \beta_y z} = \frac{11.968 \times 10^6}{0.95 \times 440 \times 136.8} = 224.61$$

Provide $7 \phi 377 \text{ mm}$

continuous

$$M = \frac{w \times l^2}{20} = \frac{0.033 \times 17 \times 4^2}{20} = 8.976$$

$$d = 144$$

$$k = \frac{M}{bd^2k_u} = \frac{8.976 \times 10^6}{1000 \times 144^2 \times 25} = \cancel{0.0173} \quad 0.0173$$

$$z = 0.5 + \sqrt{0.25 - \frac{k}{0.9}} = 0.5 + \sqrt{0.25 - \frac{0.0173}{0.9}} = 0.83$$

$$z = z \cdot d = \cancel{0.83} \times 0.83 \times 144 = 119.52$$

$$A_s = \frac{M}{0.95 f_y z} = \frac{\cancel{8.976 \times 10^6}}{1000 \times 119.52 \times 25} = \frac{8.976 \times 10^6}{0.95 \times 410 \times 119.52} = 192.81 \text{ mm}^2$$

provide $\phi 12 @ 377 \text{ mm}$

long span
Mid

$$d = d(\text{short span}) - \text{steel thickness} = 144 - 12 = 132 \text{ mm}$$

$$M = B r u l^2 \alpha_c = 0.037 \times 17 \times 4^2 = 10.064$$

$$k \rightarrow \frac{M}{b d^2 f_{cu}} = \frac{10.064 \times 10^6}{1000 \times 132^2 \times 25} = 0.0231$$

$$\alpha_c = 0.5 + \sqrt{0.25 - \frac{k}{1.9}} = 0.97 > 0.95 = 0.95$$

$$z = \alpha_c \cdot d = 0.95 \times 132 = 125.4$$

$$A_s = \frac{M}{0.95 f_y z} = \frac{10.064 \times 10^6}{0.95 \times 125.4 \times 410} = 206.04$$

provide y_{12} at 377 mm

long span continuous

$$d = 132 \text{ mm}$$

$$M = B r u l^2 \alpha_c = 0.028 \times 17 \times 4^2 = 7.616$$

$$k = \frac{M}{b d^2 f_{cu}} = \frac{7.616 \times 10^6}{1000 \times 132^2 \times 25} = 0.017$$

$$\alpha_c = 0.5 + \sqrt{0.25 - \frac{k}{1.9}} = 0.98 > 0.95 = 0.95$$

$$z = \alpha_c \cdot d = 125.4$$

$$A_s = \frac{M}{0.95 f_y z} = \frac{2.616 \times 10^6}{0.95 \times 410 \times 125.4} = 155.93$$

prov. $y_{12} @ 377 \text{ mm}$

Deflection check

$$k_s = \frac{2}{3} \text{ Pyl B } \frac{A_{sreq}}{A_s \text{ provided}}$$

$$k_s = \frac{2}{3} \times 250 \times 1 \times \frac{224.61}{377} = 99.3$$

$$m_{ik} = 0.55 + \frac{477 - 99.3}{120 \left(0.9 + \frac{11.968 \times 10^6}{1000 \times 1144^2} \right)} = 2.68 > 2$$

2.

$$d_{req} = \frac{4 \times 1000}{2 \times 26} = 76.92 = \text{OK}$$