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Civil Engineering

18/Eng03/059 // Structural Design I (eve

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ASSIGNMENT 1

1a. State the Purpose of Structural design.

a, To design for Safety

b, The design can be economical

c, The deformation of the structure must not increase with the intensity of the structure

1b. Limit State :- design consider the disadvantages of load factor design and any other failure that can cause the structure to be structurally ~~limit~~

While Elastic design :- is a method of analysis which the design of a structural member is based on a linear stress-strain relationship, assuming that the working stress are only a function of the elastic limit of the material.

STAIR CASE Design

$$\text{Slope Factor} = \frac{\sqrt{R^2 + T^2}}{T} = \frac{\sqrt{150^2 + 275^2}}{275} = 1.14$$

* load Analysis

$$\begin{aligned} \text{A. WAIST} &= R \times 24 \text{ kN/m}^2 \\ &= 0.15 \times 24 = 3.6 \text{ kN/m}^2 \end{aligned}$$

$$\text{B. FINISHES} = 1.2 \text{ kN/m}^2$$

$$\begin{aligned} \text{C. STEPS} &= T \times \frac{1}{2} \times 24 \text{ kN/m}^2 \\ &= 0.275 \times 0.5 \times 24 = 3.3 \text{ kN/m}^2 \end{aligned}$$

$$\begin{aligned} \text{D. G.K} &= (A+B) \times S_f + C \\ &= (4.8 \times 1.14) + 3.3 \\ &= 8.77 \text{ kN/m} \end{aligned}$$

$$\begin{aligned} \text{D.L, F} &= 1.4 \text{ G.K} + 1.6 \text{ Q.K} \\ &= 1.4(8.77) + 1.6(1.5) \\ &= 14.68 \text{ kN/m}^2 \end{aligned}$$

$$\begin{aligned} \text{Span} &= T_{\text{total}} + 0.5(l_a + l_b) = (275 \times 12) + 0.5(\\ &225 + 225) = 3.525 \text{ m} \end{aligned}$$

$$\begin{aligned} d &= h - \text{cover} - \frac{1}{2}\phi \\ &= 150 - 25 - 6 = 119 \text{ mm} \end{aligned}$$

$$M = \frac{F l^2}{10} = \frac{14.68 \times 3.525^2}{10} = 18.24 \text{ kN/m}$$

$$k = \frac{M}{b d^2 f_{cr}} = \frac{18.24 \times 10^6}{1000 \times 119^2 \times 25} = 0.052$$

$$I_a = 0.5 + \sqrt{0.25 - \frac{k}{0.9}} = 0.5 + \sqrt{0.25 - \frac{0.05^2}{0.9}} = 0.938$$

$$Z = I_a d = 0.938 \times 119 = 111.622 \text{ mm}$$

$$A_s = \frac{M}{0.95/yz} = \frac{18.24 \times 10^6}{0.95 \times 410 \times 111.622} = 419.53$$

$$A_{s \text{ prov}} = 452 \text{ mm}^2$$

Provide $\phi 12 @ 250 \text{ c/c}$ ($A_{s \text{ prov}} = 452 \text{ mm}^2$)

Deflection Check

$$f_s = \frac{2}{3} \times \frac{1}{\beta} \times \frac{A_{\text{req}}}{A_{\text{prov}}} \times F_y V$$

$$f_s = \frac{2}{3} \times 1 \times \frac{419.53}{452} \times 250 = 154.69 \text{ N/mm}^2$$

$$M \& F = 0.55 + \frac{477 - 154.69}{120 \left(0.9 + \frac{18.24 \times 10^6}{1000 \times 114^2} \right)}$$

$$d_{\text{req}} = \frac{\text{Span}}{m \& f \& c \& r} = \frac{3525}{1.78 \times 26} = 76.17 \text{ mm}$$

Since $d_{\text{req}} < d$, deflection is **OK**

$$2a \quad P_1 = P_2 = P_3 = \frac{4300}{4000} = 1.075 < 2 = \text{2 way Slab}$$

$$P_2 = P_8 = P_9 = \frac{4500}{4000} = 1.125 < 2 = \text{2 way Slab}$$

$$P_4 = P_5 = P_6 = \frac{4300}{4000} = 1.075 < 2 = \text{2 way Slab}$$

$$P_{10} = P_{11} = P_{12} = \frac{4000}{1500} = 2.666 > 2 = \text{1 way Slab}$$

2b) Designing Lx P2

$$\frac{L_y}{d_x} = \frac{4300}{4000} = 1.075 = \approx 1.1$$

Shortspan Coefficient = -

$$0.054$$

Longspan Coefficient = 0.058

$$0.044$$

Storing Specifications of Slab thickness = 175mm

$$f_{cy} = 25 \text{ N/mm}^2$$

$$f_y = 410 \text{ N/mm}^2$$

$$1) L = 1.46k + 1.6Q_k$$

$$Q_k = \text{weight of slab} = 0.175 \times 24$$

$$\text{Partition} = 1.0$$

$$\text{Finishes} = \frac{1.2}{6}$$

$$6.41 \text{ kN/m}^2$$

Assuming for Factory

$$Q \cdot L = (1.4 \times 6.4) + (1.6 \times 5) = 16.96 \approx 17 \text{ kN/m}^2$$

$$\text{Short Span Coefficient} = 0.044$$

$$0.033$$

$$\text{Long Span Coefficient} = 0.039$$

$$0.028$$

$$\text{Short Span mid} = p$$

$$m = B \alpha w l^2 \alpha = 0.044 \times 17 \times 4^2$$

$$= 11.968$$

$$d = h - \text{Cover} = \frac{1}{2} \phi = 144$$

$$\frac{m}{b d^2 f_{cu}} = \frac{11.968 \times 10^6}{1000 \times 144^2 \times 25} = 0.023$$

$$1000 \times 144^2 \times 25$$

$$k = z_{ca} = 0.5 + \sqrt{0.25 - \frac{k}{\sigma} a} = 0.97 > 0.95 = 0.95$$

$$z = \beta_a d = 0.95 \times 144 = 136.8$$

$$A_s = \frac{m}{0.95 f_y z} = \frac{11.968 \times 10^6}{0.95 \times 410 \times 136.8} = 224.61$$

$$0.95 f_y z$$

$$0.95 \times 410 \times 136.8$$

provide $y 12 @ 377 \text{mm}$

Continuous

$$m = B \alpha w l^2 \alpha = 0.033 \times 17 \times 4^2 = 8.976$$

$$d = 144$$

$$h = \frac{m}{bd^2 f_{cu}} = \frac{8.976 \times 10^6}{1000 \times 144^2 \times 25} = 0.0173$$

$$z_a = 0.5 + \sqrt{0.25 - \frac{k}{2.9}} = 0.5 + \sqrt{0.25 - \frac{0.0173}{0.9}}$$

$$z = z_a \cdot d = 0.83 \times 144 = 119.52$$

$$A_s = \frac{M}{0.95 f_y z} = \frac{8.976 \times 10^6}{0.95 \times 410 \times 119.52} = 192.81 \text{ mm}^2$$

Provide $y12 @ 377 \text{ mm}$

Long Span

$m \cdot d$

$$d = d_a (\text{Shortspan}) - \text{steel thickness} = 144 - 12 = 132 \text{ mm}$$

$$M = B \alpha l^2 \kappa = 0.037 \times 17 \times 4^2 = 10.664$$

$$k = \frac{M}{bd^2 f_{cy}} = \frac{10.664 \times 10^6}{1000 \times 132^2 \times 25} = 0.0231$$

$$z_a = 0.5 + \sqrt{0.25 - \frac{k}{2.9}} = 0.97 > 0.95 = 0.95$$

$$z = z_a \cdot d = 0.95 \times 132 = 125.4$$

$$A_s = \frac{m}{0.95 f_y z} = \frac{10.664 \times 10^6}{0.95 \times 125.4 \times 410} = 206.04$$

$$0.95 f_y z = 0.95 \times 125.4 \times 410$$

Provide $y12 @ 377 \text{ mm}$

Long Span Continuous

$$d = 132 \text{ mm}$$

$$m = B \alpha w l^2 \alpha = 0.028 \times 17 \times 4^2 = 7.616$$

$$k = \frac{m}{b d^2 f_{cu}} = \frac{2.616 \times 10^6}{1000 \times 132^2 \times 25} = 0.017$$

$$\text{Lar } 0.5 + \sqrt{0.25 + \frac{k}{0.9}} = 0.98 > 0.95 = 0.95$$

$$z = \beta a - d = 125.4$$

$$A_s = \frac{m}{0.95 f_{yz}} = \frac{2.616 \times 10^6}{0.95 \times 410 \times 125.4}$$

$$= 120 \text{ mm}^2$$

Provide $y 12 @ 377 \text{ mm}$

Deflection Check

$$f_s = \frac{2}{3} P_{yr} B \frac{A_{req}}{A_s \text{ provided}}$$

$$f_s = \frac{2}{3} \times 250 \times 1 \times \frac{224.61}{377} = 99.3$$

$$m_{IR} = 0.55 + \frac{477}{2} = 99.3$$

$$120 \left(0.9 + \frac{11.968 \times 166}{1000 \times 144} \right) = 2.68 > 2$$

$$d_{req} = \frac{4 \times 1000}{2 \times 26} = 76.92 = \text{OK}$$