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1a purpose of structural design

- 1) every structure to be design must be safe,
- 2) the structure must be economical
- 3) the deformation of a structure must not impere with the integrity of the structure

1.b) limit state design consider all the disadvantages of modular ratio / elastic design and load factor of design and any other failure that can cause the structures to be structural unfit for use.

UTILISE

Elastic design is a method of analysis where the ^{design} ~~sign~~ of a structural member is based on a linear stress - strain relationship, assuming that the working stress are only a fraction of the elastic limit of the material.

1c straight flight (STAIR CASE) design.

$$f_y = 410 \text{ N/mm}^2, f_{cu} = 25 \text{ N/mm}^2$$

$$\text{rise} = 150 \text{ mm}, \text{tread} = 275 \text{ mm}$$

$$\text{slab thickness} = 150 \text{ mm.}$$

$$\text{Slope factor} = \frac{\sqrt{R^2 + T^2}}{T} = \frac{\sqrt{150^2 + 275^2}}{275} = 1.14$$

load analysis:

$$\text{A - WAIST} = R + 24 \text{ kN/m}^2 \\ = 0.15 + 24 = 3.6 \text{ kN/m}^2$$

$$\text{B - FINISHERS} = 1.2 \text{ kN/m}^2$$

$$\text{C - STEPS} = T + \frac{1}{2} \times 24 \text{ kN/m}^2 \\ = 0.275 + 0.5 + 24 = 3.3 \text{ kN/m}^2$$

$$\text{D.G.K} = (A+B) \times S_F + C \\ = (4.8 \times 1.14) + 3.3 = 8.77 \text{ kN/m}$$

$$\text{D.L.F} = 14 \times \text{G.K} + 1.60 \text{ k} \\ = 1.4(8.77) + 1.6(0.5) = 14.68 \text{ kN/m}^2$$

$$\text{Span} = \text{Frontal} + 0.5(l_a + l_b) = (275 + 12) + 0.5(225 + 225) \\ = 3.525 \text{ m}$$

$$d = h - \text{Cover} - \frac{1}{2}\phi \\ = 150 - 25 - 6 = 119 \text{ mm}$$

$$M = \frac{FL^2}{10} = \frac{14.68 + 3.525^2}{10} = 18.24 \text{ kNm}$$

$$K = \frac{M}{bd^2 f_{ck}} = \frac{18.24 \times 10^8}{1000 \times 119^2 \times 25} = 0.050$$

$$I_a = 0.5 + \sqrt{0.25 - \frac{K}{0.9}} = 0.5 + \sqrt{0.25 - \frac{0.05^2}{0.9}} = 0.938$$

$$z \geq I_{ad} = 0.938 + 119 = 111.622 \text{ mm}$$

$$A_s = \frac{M}{0.95 f_y z} = \frac{18.24 \times 10^6}{0.95 \times 410 \times 111.622} = 419.58 \text{ mm}^2$$

$$A_{spnu} = 452 \text{ mm}^2$$

provided η_{12} @ 259 c/c ($A_{spnu} = 452 \text{ mm}^2$)

Deflection check.

$$f_s = \frac{2}{3} + \frac{1}{\beta} + \frac{A_{req}}{A_{prov}} \times f_{yv}$$

$$f_s = \frac{2}{3} + 1 + \frac{419.53}{452} + 250 = 154.69 \text{ N/mm}^2$$

$$M.F = 0.55 + \frac{477 - 154.69}{120 \left(0.9 + \frac{13.24 \times 10^6}{1000 \times 1192} \right)} = 1.78$$

$$d_{req} = \frac{\text{span}}{M.F + 26} = \frac{3525}{1.78 + 26} = 76.17 \text{ mm}$$

since $d_{req} < d$, Deflection is ok

Q9

A				
B	P ₁	P ₇	P ₇	P ₄
C	P ₂	P ₈	P ₈	P ₅
D	P ₃	P ₉	P ₉	P ₆

$$P_1 = P_2 = P_3 = \frac{4300}{4000} = 1.075 < 2 = 2 \text{ way slab}$$

$$P_7 = P_8 = P_9 = \frac{4500}{4000} = 1.125 < 2 = 2 \text{ way slab}$$

$$P_4 = P_5 = P_6 = \frac{4300}{4000} = 1.075 < 2 = 2 \text{ way slab}$$

$$P_{10} = P_{11} = P_{12} = \frac{4000}{1500} = 2.60 > 2 = 1 \text{ way slab}$$

Designing for P_2

$$\frac{l_y}{l_x} = \frac{4300}{4000} = 1.075 \approx 1.1$$

$$\text{Short span co-efficient} = 0.054$$

$$\text{Long span co-efficient} = 0.058$$

Assuming s_{pe} of slab thickness = 175mm

$$f_{cu} = 25 \text{ N/mm}^2$$

$$f_y = 410 \text{ N/mm}^2$$

$$DL = 14.6 \text{ k} + 1.6 \text{ k}$$

$$R.K = \text{weight of slab} = 0.175 \times 24$$

$$\text{partition} = 1.0$$

$$\text{finishes} = 1.2$$

$$\underline{\underline{0.4 \text{ k/m}^2}}$$

Assuming for factory

$$D.L (1.4 \times 6.4) + (1.6 \times 5) = 16.96 \approx 17 \text{ kN/m}^2$$

$$\text{Short span Co-efficient} = \frac{0.044}{0.033}$$

$$\text{Long span Co-efficient} = \frac{0.037}{0.028}$$

$$M = \beta \times w l^2 \alpha = 0.044 \times 17 \times 4^2 = 11.968$$

$$d = h - \text{cover} = \frac{1}{2} \phi = 144 \text{ mm}$$

$$k = \frac{M}{bd^2 f_{cu}} = \frac{11.968 \times 10^6}{1000 \times 144^2 \times 25} = 0.023$$

$$\bar{z} = 0.5 + \sqrt{0.25 - \frac{k}{0.9}} = 0.97 > 0.95 = 0.9$$

$$z = \bar{z} d = 0.95 \times 144 = 136.8$$

$$A_s = \frac{M}{0.95 f_y z} = \frac{11.968 \times 10^6}{0.95 \times 410 \times 136.8} = 224.61 \text{ mm}^2$$

provide 3 @ 377 mm

Continued

$$M = \beta \times w l^2 \alpha = 0.033 \times 17 \times 4^2 = 8.976$$

$$d = 144$$

$$k = \frac{M}{bd^2 f_{cu}} = \frac{8.976 \times 10^6}{1000 \times 144^2 \times 25} = 0.0173$$

$$I_a = 0.5 + \sqrt{0.25 - \frac{k}{69}} = 0.5 + \sqrt{0.25 - \frac{0.0173}{6.7}} = 0.83$$

$$Z = I_a d = 0.83 \times 144 = 119.52$$

$$A_s = \frac{M}{0.95 f_{yz} Z} = \frac{8.976 \times 10^6}{0.95 \times 410 \times 119.52} = 192.81 \text{ mm}^2$$

provide $y_{12} @ 397 \text{ mm}$

long span

mid

$$d = d(\text{short span}) - \text{steel thickness} = 144 - 12 = 132 \text{ mm}$$

$$M = \beta_x w l^2 \alpha = 0.037 \times 17 \times 4^2 = 10.064$$

$$k = \frac{M}{bd^2 f_{cu}} = \frac{10.064 \times 10^6}{1000 \times 132^2 \times 25} = 0.0237$$

$$I_a = 0.5 + \sqrt{0.25 - \frac{k}{69}} = 0.97 > 0.95 = 0.95$$

$$Z = I_a d = 0.95 \times 132 = 125.4$$

$$A_s = \frac{M}{0.95 f_{yz} Z} = \frac{10.064 \times 10^6}{0.95 \times 410 \times 125.4} = 206.07$$

Provide gir @ 377mm

long span continuous.

$$d = 132 \text{ mm}$$

$$M = \beta_{cs} \omega l^2 \alpha = 0.028 \times 17 \times 11^2 = 7.616$$

$$k = \frac{M}{bd^2 f_{cu}} = \frac{7.616 \times 10^6}{1000 \times 132^2 \times 25} = 0.017$$

$$I_a = 0.5 + \sqrt{0.25 - \frac{k}{0.9}} = 0.98 > 0.95 = 0.95$$
$$\Rightarrow I_{ad} = 125.4$$

$$AS = \frac{M}{0.95 f_{y2}} = \frac{7.616 \times 10^6}{0.95 \times 410 \times 125.4} = 155.93 \text{ mm}^2$$

provide gir @ 377mm

Deflection check.

$$f_s = \frac{2}{3} P_{yr} \beta \frac{A_{req}}{A_{s \text{ provided}}}$$

$$f_s = \frac{2}{3} \times 250 \times 1 + \frac{224.61}{377} = 99.3$$

$$M_i R = 0.55 + \frac{477 - 99.3}{120 \left(0.9 + \frac{11.968 \times 10^6}{1000 + 1442} \right)} = 2.68 > 2$$

$$\Delta_{req} = \frac{4 \times 1000}{2 \times 26} = 76.92 = \text{OK}$$