

- a) To design for safety
 b) The design must be economical.
 c) The deformation of the structure must not impose with the integrity structure.

Staircase Design

$$\text{Slope factor } S = \frac{\sqrt{R^2 + T^2}}{T} = \frac{\sqrt{150^2 + 275^2}}{275}$$

$$= 1.14$$

Load Analysis

$$A. \text{ weight} = R \times 24 \text{ KN/m}^3$$

$$= 0.15 \times 24 = 3.6 \text{ KN/m}^2$$

$$B. \text{ finishes} = 1.2 \text{ KN/m}^2$$

$$C. \text{ steps} = T \times \frac{1}{2} \times 24 \text{ KN/m}^2$$

$$= 0.275 \times 0.5 \times 24 = 3.3 \text{ KN/m}^2$$

$$D. \text{ G.K. } ((A+B) \times 3) + C$$

$$= (4.8 + 1.14) + 3.3$$

$$= 9.24 \text{ KN/m}$$

$$\Delta L. F = 15 \text{ kN} + 1.14 \text{ kN}$$

$$= 1.4 (3.77) + 1.6 (1.5)$$

$$= 14.68 \text{ kN/m}^2$$

$$\text{span} = T_{\text{total}} + 0.5 (1.07)h = (2.75 \times 12) + 0.5$$

$$(225 + 225) = 3.525 \text{ m}$$

$$d = h - \text{cover} - \frac{1}{2} \phi$$

$$= 150 - 25 - 6 = 119 \text{ mm}$$

$$m = \frac{f_l^2}{10} = \frac{14.68 \times 3.525}{10} = 18.24 \text{ kNm}$$

$$k = \frac{m}{bd^2 f_{ck}} = \frac{18.24 \times 10^6}{1000 \times 119^2 \times 25}$$

$$= 0.052$$

$$I_a = 0.5 + \sqrt{0.25 - \frac{k}{24}} = 0.5 + \sqrt{0.25 - \frac{0.052}{0.9}}$$

$$= 0.938$$

$$Z = I_a d = 0.938 \times 119 = 111.622 \text{ mm}$$

$$A_s = \frac{m}{0.95 f_y Z} = \frac{18.24 \times 10^6}{0.95 \times 410 \times 111.622} = 419.52$$

$$A_s \text{ for } = 419.52 \text{ mm}^2$$

Provide 4 ϕ @ 2590k (area = 419.52)

Deflection check

$$f_{cr} = \frac{2}{3} \times \frac{l}{\beta} \times \frac{Area}{A_{ps}} \times f_{yp}$$

$$f_{cr} = \frac{2}{3} \times 1 \times \frac{419.52}{452} \times 250$$

$$= 154.69 \text{ N/mm}^2$$

$$m \cdot f = 0.98 + 477 - 154.69$$

$$20 \left(0.9 + \frac{18.24 \times 10^6}{1000 \times 119^2} \right) \approx 1.78$$

$$d_{req} = \frac{\text{span}}{m \cdot f \times ed} = \frac{3525}{1.78 \times 26} = 76.17 \text{ mm}$$

Since $d_{req} < d$, Deflection is OK.

2a.) $P_1 = P_2 = P_3 = \frac{4300}{4000} = 1.075 < 2$
 $= \text{2 way slab}$

$P_2 = P_3 = P_4 = \frac{4500}{4000} = 1.125 < 2$
 $= \text{2 way slab}$

$P_4 = P_5 = P_6 = \frac{4300}{4000} = 1.075 < 2$
 $= \text{2 way slab}$

$P_7 = P_{11} = P_{12} = \frac{4000}{1500} = 2.66 > 2$
 $= \text{1 way slab}$

2b.) Designing for P_2

$\frac{l_y}{l_x} = \frac{4300}{4000} = 1.075 \approx 1.1$

short span coefficient = $\frac{1}{0.094}$

long span coefficient = $\frac{0.058}{0.044}$

Assuming specification of slab thickness = 175mm

$$f_y = 410 \text{ N/mm}^2$$

$$D.L = 1.4 \text{ B.K} + 1.6 \text{ B.K}$$

$$G.K = \text{weight slab} = 0.175 \times 24$$

$$\text{partition} = 1.0$$

$$\text{finishes} = 1.2$$

$$6.4 \text{ kN/m}^2$$

Assuming for factory

$$D.L = (1.4 \times 6.4) + (1.6 \times 5)$$

$$= 16.96 \approx 17 \text{ kN/m}^2$$

$$\text{Short span coefficient} = 0.044$$

$$0.033$$

$$\text{Long span coefficient} = 0.032$$

$$0.028$$

$$\text{Short span } m^2 = P$$

$$m = P \times w l^2 \times \alpha = 0.044 \times 17 \times 4^2$$

$$= 11.968$$

$$d = h - \text{cover} - 1/2 \phi = 144$$

$$A_s = \frac{m}{1000 \times 144^2 \times 24}$$

all for

provide y_{12} at = 377mm

long span Continuous

$$d = 1320\text{mm}$$

$$m = \frac{P_{max} l^2}{8} = 0.028 \times 17 \times H^2 = 7.616$$

$$k = \frac{m}{bd^2 f_{cu}} = \frac{7.616 \times 10^6}{1000 \times 132^2 \times 25} = 0.017$$

$$z = 0.5 + \sqrt{0.25 + \frac{k}{0.9}} = 0.98 > 0.95 = 0.95$$

$$z = z_{max} d = 125.4$$

$$A_s = \frac{m}{0.95 f_y z} = \frac{7.616 \times 10^6}{0.95 \times H_{10} \times 125.4} = 155.93$$

provide y_{12} @ 377mm

Deflection check

$\frac{1}{250} f_y$ B $\frac{1}{250}$ $\frac{1}{250}$
As provided

$$A_s = \frac{M}{0.95 f_{yk}} = \frac{8.976 \times 10^6}{0.95 \times 410 \times 119.52} = 192.81 \text{ mm}^2$$

provide y_{12} @ 377mm

long span

m.c.d

$$d = d(\text{short span}) - \text{steel thickness} = 144 - 12 = 132 \text{ mm}$$

$$m = \frac{B \alpha w l^2}{2} = 0.037 \times 17 \times 4^2 = 10.064$$

$$k = \frac{M}{b d^2 f_{cu}} = \frac{10.064 \times 10^6}{1000 \times 132^2 \times 25} = 0.0231$$

$$R_a = 0.5 + \sqrt{0.25 - \frac{k}{0.9}} = 0.97 > 0.95 = 0.95$$

$$Z = R_a \cdot d = 0.95 \times 132 = 125.4$$

$$A_s = \frac{M}{0.95 f_{yk} Z} = \frac{10.064 \times 10^6}{0.95 \times 125.4 \times 410} = 206.04$$

$$= 0.023$$

$$R_a = 0.5 + \sqrt{0.25 - \frac{k}{0.9}} = 0.977 \approx 0.98$$

$$= 0.98$$

$$Z = R_a \cdot d = 0.98 \times 144 = 136.8$$

$$A_s = \frac{M}{0.95 R_y Z} = \frac{11.968 \times 10^6}{0.95 \times 410 \times 136.8} = 224.61$$

provide $\phi 12 @ 377 \text{ mm}$

Continuous

$$M = \beta w l^2 x = 0.033 \times 17 \times 4^2 = 8.976$$

$$d = 144$$

$$k = \frac{M}{b d^2 f_{cu}} = \frac{8.976 \times 10^6}{1000 \times 144^2 \times 25} = 0.0173$$

$$R_a = 0.5 + \sqrt{0.25 - \frac{k}{0.9}} = 0.5 + \sqrt{0.25 - \frac{0.0173}{0.9}} = 0.83$$

$$Z = R_a \cdot d = 0.83 \times 144$$

$$= 119.52$$

$$f_d = \frac{2}{3} \times 250 \times 1 \times \frac{224.61}{377}$$

$$= 99.3$$

$$MIR = 0.53 \times 477 = 99.3$$

$$120 \left(0.9 + \frac{11.968 \cdot 10^6}{1000 \times 1111^2} \right)$$

$$= 2.68 > 2$$

$$= 2$$

$$d_{req} = \frac{H \times 1000}{2 \times 26}$$

$$= 26.92 = OK.$$